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Increase in surface runoff in the central mountains of Mexico: lessons from the past and predictive scenario for the next century

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Abstract

The hydrological response of a medium scale mountainous watershed (Mexico) is analysed over half a century. The hydrograph separation highlights an increasing surface runoff contribution since the early 1970's. This increase is attributed to land use changes while the meteorological forcing (rains) remains statistically stable over the same period. As a consequence, the intensity of annual extreme floods has tripled up over the period of survey, increasing flood risks in the region. The paper ends with a climatic projection over the 21st century. The decrease of precipitation and the increase of temperature should accentuate the trend engaged since the 1970's by reducing groundwater resources and increasing surface-runoff and associated risks.

1 Introduction

Over the last decades, Mexico has suffered from degradation of its surface water bodies which is imposing undeniable economical costs (Alcocer and Escobar, 1993). Nowadays Mexican water resources are commonly considered poor in quality and sparse in quantity (Vidal et al., 1985), a situation exemplified in Michoacán state. A recent study led by various local institutions (Ortiz Ávila, 2009) stressed that water contamination, solid residuals management and drinkable water are the main environmental priorities of the Michoacán settlements. In terms of quantity, the state has experienced a decrease of about 70% in its surface water resources over the last century (Vargas Uribe, reported by Morales, 2007). This evolution has been correlated with the high emigration rate in Michoacán (63% of total population), which contributed to the main soils and water resources use changes (López-Granados et al., 2006).

The present paper aims at investigating water cycle changes from 1956 to 2001 in the Cointzio watershed (Michoacán state), a medium scale mountainous basin representative of the Central Transvolcanic Mexican Belt.

Our approach considers the impact of climate and human-induced environmental

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changes on water runoff in the watershed. To that purpose a hydro-meteorological database running from 1956 to 2001 was employed following the latter methodology: available rain and water discharge series were gathered (and digitalized when necessary) and criticized (missing data and quality control) to provide reliable series. A particular effort was paid to estimate the accuracy of the historical water discharge data in relation with sampling frequency (Sect. 3). In a second time, hydrograph separation technique was applied to water discharge records to define the baseflow/surface-runoff ratio and its evolution over decades. The generated time series were tested with various statistical methods in order to provide accurate hydro-climatic trends and objective interpretations (Sect. 4). Finally, a climate model was applied to five meteorological stations to go towards scenarios of evolution for the coming century (Sect. 5).

2 Study area

The Cointzio watershed is located in the hydrological region of Lerma-Chapala, within the Central Transvolcanic Mexican Belt, in the state of Michoacán (Fig. 1). It drains a surface of about 650 km², ended by the Cointzio reservoir (4 km², 65 Mm³). The latter provides about 22% of drinking water distributed in Morelia, capital of the state situated 13 km downstreams. Water demand of the city has been growing over the last decades because of increasing individual water consumption coupled with a severe urban growth: Morelia experienced an augmentation of its population of 600% during the period 1975–2000 (López-Granados et al., 2001) and counts now over 700 000 inhabitants (INEGI, 2006).

The climate of the region is temperate sub-humid, characterized by a rainy season from May to October and a dry season the rest of the year (Rubio, 1998). Mean annual rainfall is 770 mm in Morelia, ranging from 400 to 1100 mm/y in thirty years (period 1975–2005) (Carlón Allende and Mendoza, 2007). These meteorological conditions imply that 77% of the drainage system consists of temporary watercourses active only during rainy season (Susperregui et al., 2009). The main river of the watershed is

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the Rio Grande de Morelia whose source lies about 25 km upstream of the Cointzio reservoir.

The Cointzio basin is underlain by igneous rocks (both lavas and pyroclasts) originating from Quaternary volcanic activity. Soils and landforms developed in most of the watershed have been derived from these volcanic materials (Carlón Allende et al., 2009). Such soils present susceptibility to erosion (Poulenard et al., 2001). They are mainly Andisol in the headwater areas, Acrisol in the hillsides and Luvisol in the plain (INEGI, 2002).

In 2000, major land cover and land uses were scrublands (23.7%), forests (19.6%), rain fed agricultural lands (18.5%), irrigated cultures (15.5%) and grassland (6.3%) (López-Granados et al., 2001). According to these uses, the basin can be classified as agro-sylvo-pastoral.

Using GIS to represent data of land cover and land use, Mendoza and López-Granados (2007) were able to identify the major changes in the Cointzio watershed over the period 1975–2000. The main changes in surface were increase of scrublands (9.6%), recovery of forests (6.2%), deforestation (5.5%), degradation of forests (4.1%) and urbanization (1.3%). The major part of these changes occurred during the 1986–1996 period.

3 Data and methods

3.1 The hydrometeorological historical database and its limitation

Rain series were extracted from the Mexico Climatological Station Network Data (CLICOM). This database reports all of the meteorological stations surveyed by the Servicio Nacional de Meteorología de México. A pre-analysis was realized by Vinson (2008). Among the eighty meteorological stations in the state of Michoacán, five were considered of interest. Three are located within the Cointzio watershed, namely the stations of Acuitzio del Canje (2020 m), Santiago Undameo (2020 m) and Cointzio (2000 m) (Ac,

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SU and C in Fig. 1). The stations of Jesus del Monte (2180 m) and Morelia (1920 m) that are close to the watershed delimitation were also taken into account (JdM and M in Fig. 1). The first one is of interest to examine potential orographic effects and the second one because of its reliability and local importance.

5 The quality of time series has been examined and is summarized in Table 1. All years presenting missing values have been rejected as well as years presenting evident data capture errors. The station of Acuitzio del Canje (Ac) is the only one showing significant gaps during the whole period. For a reason of clarity, results presented in Sect. 4.1 focus on the precipitation series of Santiago Undameo and Cointzio (Fig. 2a). These
10 latter are the only consistent time series within the watershed and the five series exhibit a very similar pattern (for details see Vinson, 2008).

The gauging station of Santiago Undameo (SU in Fig. 1) constitutes the ultimate control upstream the reservoir of Cointzio: it drains 628 km². Its monitoring has been launched in 1939, year of construction of the reservoir of Cointzio. A Parshall flume
15 was built, allowing a control of the hydraulic section, and a stage-discharge rating curve was established by the Comisión Nacional del Agua (CNA). The monitoring ended in 2001.

The 1939–1985 period was previously digitalized by the CNA (BANDAS database), while the 1985–2001 period was digitalized as part of our investigations. Although
20 overall database covers the 1939–2001 period, we decided to focus on 1956–2001 series since the first flume was destroyed during a major event in 1953. The re-building and its calibration ended during 1955. The water discharge database is presented in Fig. 2b. Some minor missing period events were identified in 1975, from 17–20 October because of a flood event. Four days of missing data are reported in 1992 and one more
25 in 1997. With a period of missing data of several weeks in September and October, the year 1998 could not be considered in our analysis.

From the beginning of 2008, the station has been updated to estimate instantaneous water and sediment discharges. Water level was surveyed at a five minute time-step with a Thalimede OTT water-level gauge. Water discharge time-series were deter-

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mined through the stage-discharge rating curve. Our aim was to take advantage of this high-frequency monitoring to evaluate the accuracy of daily discharge values provided by historical time-series, and thus to carry out a long-term analysis without misinterpretation.

5 Daily discharge data was historically deduced by the CNA from a minimum of three manual measures (respectively at 06:00 LT, 12:00 LT and 18:00 LT). Taking into account the occurrence of flash flood events in the basin, the historical sampling frequency is questionable. To validate the methodology, a sub-sampling of the real-time-series acquired in 2008 was generated. Resulting mean daily discharges were then compared to
10 reference mean daily discharges derived from the high-frequency time-series (Fig. 3).

Reference discharge values and sub-sampled ones exhibit a strong linear relationship (Fig. 3a). Distribution of the relative error presented in Fig. 3b exhibits a Gaussian shape, centred on zero and characterized by a low standard deviation of about 7%. Such pattern demonstrates the reliability of the historical sampling. Loss of accuracy
15 remains very limited if focusing on short-term dynamics as well as on seasonal water budgets calculation (maximum under-estimation of 2% of reference volume). The further processing of 1956–2001 historical time-series is thereby validated.

3.2 Hydrograph separation technique

We aimed to consider the effects that may have induced an alteration of physical characteristics of the watershed, such as land use change, on the hydrological cycle. We
20 subsequently focused on the characterization of annual baseflow/surface-runoff ratio and its evolution over the past fifty years at Santiago Undameo.

Stream flow hydrographs were separated into Annual Base Flow (ABF) and Annual Surface-Runoff (ASR) components. The baseflow component has traditionally been
25 associated with groundwater discharging into the stream and the surface-runoff component with precipitation that enters the stream as overland runoff (Sloto and Crouse, 1996; Chapman, 1999). The aim of this paper is not to work on the physically based concept of hydrograph separation; the technique was applied as a tool for detecting

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trends in water discharge behaviour. The reader can refer to Chapman (1999) and Nathan and McMahon (1990) to get an overview of techniques commonly used by engineers to quantify the baseflow contribution of a watershed.

A variant of a hydrograph separation method was programmed using Matlab® software. The algorithm used is based on the concept developed by Pettyjohn and Henning (1979) and is commonly referred as the “smoothed minima technique” in the literature (Brodie and Hostetler, 2005). This method can be described as connecting with straight lines the minima of fixed intervals τ of the hydrograph. The sequence of these connecting lines defines the baseflow hydrograph. In case of missing data, a mean value calculated from nearest neighbours was used. Calculated Base Flow delimitation (CBF) was thus obtained as follows:

$$CBF(d, \tau) = \min_{d \in [d - \frac{\tau}{2}, d + \frac{\tau}{2}]} \overline{Q}_d(d) \quad (1)$$

With d : day of the year

τ : interval parameter (in days and necessarily pair)

$\overline{Q}_d$: mean daily water discharge (in $\text{m}^3 \cdot \text{s}^{-1}$)

Annual Base Flow (ABF) and Annual Surface Runoff (ASR) were then estimated by:

$$ABF = \sum_{d=1}^{365} CBF(d) \quad (2a)$$

$$ASF = \sum_{d=1}^{365} \overline{Q}_d - ABF \quad (2b)$$

In our analysis, we tested a broad range of τ values from four to fourteen days. From a general point of view, it is clear that CBF (and thus ABF) will decrease for increasing τ .

Such behaviour is illustrated in Fig. 4 for two contrasted hydrological years that occurred in 1993 and 1994. Year 1993 (Fig. 4a) was hydrologically active with many

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significant flooding periods lasting several days. CBF function decreases significantly for increasing τ values from six to fourteen days. Conversely, in the case of year 1994 (Fig. 4b), periods with high water discharge were scarce and never exceeded a couple of days. As expected, CBF function does not vary significantly with τ .

To determine whether a large τ value (fourteen days or more) or a low one (six days or less) is more appropriate, it is necessary to introduce a physically based analysis. Let's imagine a theoretical Dirac precipitation uniformly distributed on the watershed. The water surface runoff increases rapidly at the outlet before its recession. The baseflow reacts slowly and its level increases slightly (Fig. 5a, first day). Considering an independent event occurring the following day, the watershed reacts similarly (Fig. 5a, second day). Since historical database only provides some mean daily discharges (circle, cross, square and triangle in Fig. 5a), the estimation of baseflow level requires a minimal period of one day without significant precipitation. In that case:

$$Q_{\text{inst}} \approx \text{cte} \quad (3a)$$

$$\overline{Q}_d = \frac{1}{N} \sum_{i=1}^N \text{cte} \approx Q_{\text{inst}} \quad (3b)$$

With Q_{inst} : instantaneous water discharge (in $\text{m}^3 \cdot \text{s}^{-1}$),

N : number of samples per day.

The real situation occurring in September 1993 illustrates perfectly the concept. It is presented in Fig. 5b as part of the series presented in Fig. 4a. Significant rainfalls occurred at the meteorological stations of Cointzio and Santiago Undameo until 17 September (black and grey bars in Fig. 5b) and were followed by a four days dry period. The hydrological response follows this meteorological forcing with a delay of one to two days. In that case, it is clear that baseflow level is reached during the period extending from 19–23 September.

By examining the precipitation database at our disposal, it appeared that the longest time-interval required to reach a day without significant rainfall event ($P_d < 5 \text{ mm}$) is

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approximately of five days. The upper discussion highlights that a τ interval of six days is best suitable in Fig. 4a, while coinciding with both rainfall pattern and hydrograph visual examination.

However, to prevent any misinterpretation, the algorithm was also run with both a shorter (four days) and a longer (ten days) τ value.

3.3 Trends detection from statistical methods

Statistical significance of gradual trends was detected by applying the rank-based Mann-Kendall test (Mann, 1945; Kendall, 1975) and magnitude of trends was estimated from Sen's method (Sen, 1968).

The Mann Kendall test is a non parametric statistic that has been widely used to assess the significance of monotonic trends in hydro-meteorological time-series (e.g., Lettenmaier et al., 1994; Marengo and Tomasella, 1998; Jiang et al., 2007; Zhang et al., 2008; among others). The test assumes that there is no serial correlation in the data. Such assumption is reasonable for the rainfall and runoff records presented in this paper.

The null hypothesis H_0 is that the sample of data is independent and identically distributed. The alternative hypothesis H_1 is that a monotonic trend exists in the time-series. Mann-Kendall method was applied by considering the statistic S as:

$$S = \sum_{i=2}^n \sum_{j=1}^{i-1} \text{sign}(x_i - x_j) \quad (4)$$

Where x_i and x_j are the sequential data values, n is the length of the time-series and $\text{sign}(x_i - x_j)$ is -1 for $(x_i - x_j) < 0$; 0 for $(x_i - x_j) = 0$ and 1 for $(x_i - x_j) > 0$.

In the absence of ties, the variance $\text{Var}[S]$ of the statistic S was obtained as:

$$\text{Var}[S] = \frac{n(n-1)(2n+5)}{18} \quad (5)$$

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The standardized statistical test Z was computed by:

$$Z = \begin{cases} \frac{S-1}{\sqrt{\text{Var}[S]}} & \text{if } S > 0 \\ 0 & \text{if } S = 0 \\ \frac{S+1}{\sqrt{\text{Var}[S]}} & \text{if } S < 0 \end{cases} \quad (6)$$

Positive value of Z indicates an increasing trend while negative Z indicates a decreasing trend. When testing monotonic trends at an α significance level, H_0 was rejected for absolute value of Z greater than $Z_{(1-\alpha/2)}$, where $Z_{(1-\alpha/2)}$ is the value of the standard normal distribution with a probability of $\alpha/2$. In this work, significance level of $\alpha=0.01$ (99% confidence) was applied and null hypothesis was rejected if $|Z| > Z_{0.995}=2.575$.

Sen's method is a non-parametric statistic used in determining the presence and magnitude of a trend slope. This test proceeds by calculating the slope as a change in measurement per change in time. Trend slopes magnitudes were obtained following the method of Hirsch et al. (1982):

$$\beta = \text{median} \left[\frac{(x_j - x_i)}{(j - i)} \right] \quad \text{for all } i < j \quad (7)$$

Where x_j and x_i are data points measured at times j and i , respectively.

Mann-Kendall test and Sen's method were applied on precipitation, surface runoff and water discharge time series. Results are presented in Sect. 4.2.

4 Results

4.1 Pattern of the precipitation time-series from 1956 to 2001

Our analysis is based on the examination of three statistical indicators, namely the total annual precipitation, the maximum daily precipitation (from year to year) and the sum

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of the 5% days undergoing maximum rainfall events. The latter basically corresponds to a virtual “top eighteen days” each year.

Results presented in Fig. 6 only focus on the stations of Cointzio and Santiago Undameo but, as said previously conclusions are similar for the three other stations.

5 Annual precipitation series fluctuates significantly in the range 650–1200 mm/y; however there is no evidence of tendencies over the last fifty years. Cumulated top 5% precipitations correspond to about 400–500 mm of total annual precipitations, which means that 50% of annual volumes are precipitated in 5% of the time. Such ratio highlights the heavy rainfall pattern characterizing the region during wet season.

10 Maximal precipitation exhibits the same pattern than annual volumes and top 5% precipitations. Although maximum daily precipitation fluctuates from year to year, it basically remains stable in the range 35–60 mm and roughly corresponds to 5% of annual precipitation.

4.2 Pattern of the Annual Surface Runoff (ASR) from 1956 to 2001

15 Annual Surface Runoff (ASR) and Annual Base Flow (ABF) were calculated from 1956 to 2001 by applying Eqs. (1) and (2) with τ intervals of four, six and ten days.

As shown in Fig. 7a, ASR volume series remains globally constant over 1956–2001 period, in the range $(10\text{--}40) \times 10^6 \text{ m}^3 \cdot \text{y}^{-1}$. ABF volume is always predominant. It fluctuates in the range $(30\text{--}80) \times 10^6 \text{ m}^3 \cdot \text{y}^{-1}$. Unlike ASR, ABF volume exhibits clear inter-decades trends. It shows a continuous decrease from the beginning of the seventies to the end of the eighties that could be correlated to the dry cycle discussed by Carlón Allende and Mendoza (2007) and Metcalfe et al. (2007).

Another interesting pattern is depicted by %ASR and %ABF series presented in Fig. 7b. Unlike ASR volume, %ASR has been increasing substantially since the seventies. It remains almost constant (nearly 25%) until 1970 and then increases to reach about 40% by the end of the eighties to 2001. This significant water balance change does not depend on the hydrograph separation processing as it is observed for every τ value (grey shading in Fig. 7b). The latest values remain in a typical range of values

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encountered in North American watersheds (Neff et al., 2005).

Looking simultaneously at volumes and percentages it appears that surface runoff versus baseflow is not affected by the precipitation pattern. This is well illustrated by couples of years 1956–1957 and 1993–1994. Between years 1956 and 1957, the total water discharge rose from $39 \times 10^6 \text{ m}^3$ to $113 \times 10^6 \text{ m}^3$ (Fig. 7a) while %ASR (or %ABF) remained almost stable (28.5% to 33%, Fig. 7b). Between years 1993 and 1994, the total water discharge dropped from $153 \times 10^6 \text{ m}^3$ to $44 \times 10^6 \text{ m}^3$ while %ASR (or %ABF) did not change significantly (34% to 36%).

10 The last two paragraphs point out that the ratio ASR/ABF and its evolution over years does not depend on meteorological forcing. Hence, it is the partition of rainfall input between surface-runoff and baseflow that has changed gradually in Cointzio. Between 1956 and 2001 the watershed has been retaining less surface runoff and has been sustaining lower baseflow. This change in hydrological response is very likely to be associated with physical alterations of the watershed, such as land use change and surface impermeabilization. Consequently, %ASR evolution is hereafter considered as an indicator of the human-induced environmental changes in the watershed.

4.3 Evolution of water discharge extreme events

The goal of the present section is to quantify hydro meteorological trends and evaluate the relative impacts of both meteorological and human-induced changes on floods.

20 The Mann Kendal test was applied to annual rain (Cointzio weather station), %ASR series and water discharge. Results are presented in Table 2 and trends are depicted in Fig. 8.

As specified in Sect. 4.1, rain series does not exhibit any trend (Fig. 8a, Table 2). Hence, it can be considered that climatic changes, in terms of water input in the watershed, were insignificant during the study period. Conversely, %ASR increases significantly between 1956 and 2001. Applying Sen's method, the resulting trend slope has amplitude of 0.33, which corresponds to an increase of 69% in the contribution of surface-runoff to overall water discharge. The human-induced changes at the origin

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of the %ASR increment (see Sect. 4.2) has undoubtedly contributed to the increase in extreme floods intensity depicted in Fig. 8c. This graph displays the annual flood peak $\overline{Q_d}$ max. The series exhibits a clear increase with trend slope amplitude of 0.50. It corresponds to an increase rate of $\overline{Q_d}$ max as high as 232% over half a century. The series also shows a clear increment of the variability since 1975 but there is no doubt that flood risk has increased over years.

5 Potential evolutions over next decades

In terms of water management, it is crucial to evaluate whether the engaged trend will continue or not.

Future climate change prediction was thus conducted for the five weather stations considered. Estimates of mean annual precipitation and temperature presented in Fig. 9a, b are deduced from a spline climatic model developed for a normalized period (years 1961 to 1990, nominated “contemporary climate”) and updated with weighted outputs from a Global Circulation Model (Canadian Centre for Climate Modelling and Analysis), emission scenario A2, for the decades centred in the years 2030, 2060 and 2090 (Crookston, 2008; Saenz-Romero et al., 2009). Following Saenz-Romero et al. (2009), precipitation and temperature series are combined to provide an aridity index (ratio of the square root of annual degree days $>5^{\circ}\text{C}$ to annual precipitation) that represents the potential for moisture stresses to develop in the vegetation (Fig. 9c).

Average estimations among the five weather stations for the contemporary climate and compared to the global change scenario, indicate an expected decrement in precipitation of 15.4, 19.1 and 27.7%, and an increment in mean annual temperature of 1.6, 2.5 and 4.4°C , for the decades centred in the years 2030, 2060 and 2090, respectively.

In terms of extreme events, it would be inappropriate to associate the annual precipitation decreasing pattern with a decrement in flood intensity: flood dynamics is governed by instantaneous rainfall intensity rather than by annual budgets.

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The combination of increment of temperature and decrement of precipitation evidently causes an increment of aridity, measurable through the annual aridity index (Fig. 9c, where smaller values mean a climate colder and moister and larger values indicate a climate warmer and dryer). Since the aridity index is closely related to the type of vegetation (Rehfeldt, 2006; Rehfeldt et al., 2006), it is reasonable to expect that climate change will cause a decrement of the vegetation coverage, and consequently, an increment of runoff contribution, as previously reported by Ranzi et al. (2002) and García-Ruiz et al. (2008).

6 Conclusion and perspectives

Over half a century, significant changes have occurred in the water balance of Cointzio, a medium scale watershed representative of the mountainous highlands of Central Mexico. Surface-runoff has increased of about 70% and has consequently led to a severe increase in extreme flood events (magnitude has tripled up over 1956–2001). The main changes occurred from the 1970's and are believed to be principally driven by human-induced changes.

The state of Michoacán experienced very strong migration fluxes which contributed to the main soils and water resources use changes, as revealed by López-Granados et al. (2006) from remote sensing analysis. Within the watershed, inhabitants have been leaving countryside while Morelia (capital of Michoacán) has been growing tremendously, from 100 000 inhabitants in 1950 to 700 000 in 2005. During the same period, precipitation exhibited no statistical trend in the region, attesting the absence of significant climatic change.

Predicting in which proportion water cycle disequilibrium engaged since the 1970's will extend in the coming decades is a key issue. First, the growth in water consumption related to urban needs and new agricultural practices (avocados) will reinforce the pressure on local water resources. Secondly, substantial climate changes are likely to occur: numerical simulations presented herein indicate a drastic reduction in rain

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budget (–28% in 2090) and an increase of the aridity. These changes are expected to alter vegetation coverage and consequently, accentuate the surface-runoff contribution.

At a global scale, Bates et al. (2008) recently reported that flash-floods and inundation should become more frequent worldwide. A regional analysis of precipitation intensity would be required to further improve our understanding of flood risk in the highlands of Central Mexico.

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Table 1. Quality of the rain database for the five stations considered. A value of one corresponds to a complete time-series.

		1955–1960	1961–1970	1971–1980	1981–1990	1991–2000	2001–2010	2030–2090
within watershed	Ac	no data	0.5	1.0	0.9	0.3	0.8	simulation
	SU	1.0	1.0	0.9	0.9	0.9	0.8	
outside watershed	C	1.0	1.0	0.9	0.5	0.8	0.8	
	JdM	1.0	1.0	1.0	1.0	0.9	0.9	
	M	1.0	1.0	1.0	0.5	not at our disposal		

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Table 2. Results of Mann-Kendall and Sen's tests for runoff, extreme discharges and precipitation time-series.

	<i>S</i>	Calculated <i>Z</i> score	Critical <i>Z</i> value at $\alpha=0.01$	Significant trend at 99% confidence	Trend slope β	Increase between 1956 and 2001
%ASR 1956–2001	541	5.11	2.575	Increasing	0.33	+69%
$Q_{\sigma\max}$ 1956–2001	463	4.37	2.575	Increasing	0.50	+232%
Precipitation 1956–2006	–4	–0.04	2.575	–	–	–

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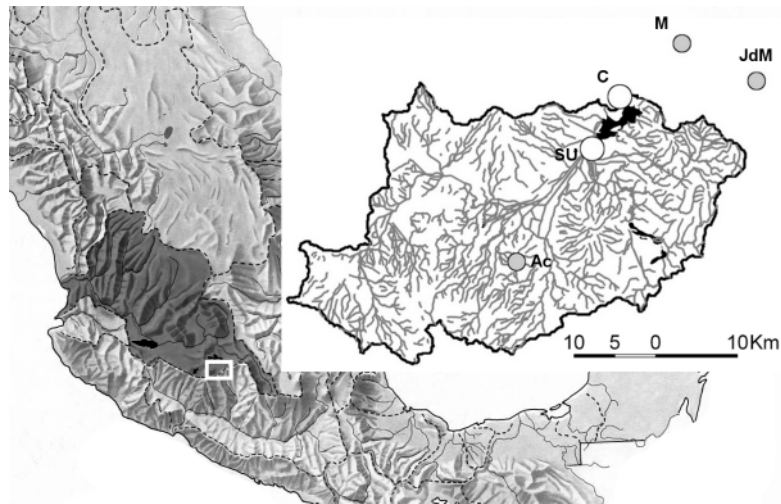


Fig. 1. Localisation of the Cointzio watershed in the Transvolcanic Mexican Belt. The darkened area delimits the Lerma-Chapala Basin system. The white box delimits the study area. Circles correspond to the location of the meteorological stations of Santiago Undameo and Cointzio (in white) and Acuitzio del Canje, Morelia and San Jesus del Monte (in grey). The black area is the reservoir of Cointzio. The general map is source of the University of Texas Libraries.

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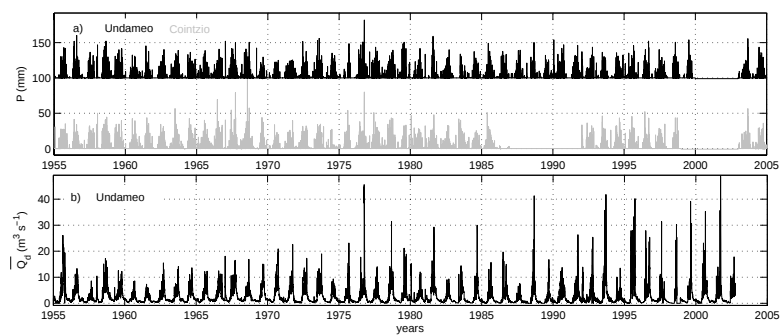


Fig. 2. Hydrometeorological time-series. **(a)** Rain series at Santiago Undameo (black line) and Cointzio (grey line). **(b)** Mean daily discharge at Santiago Undameo.

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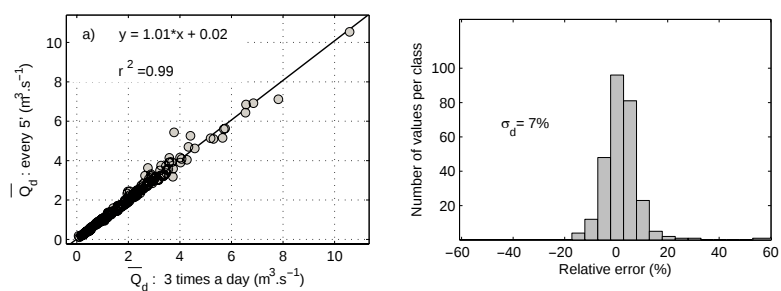


Fig. 3. (a) Inter-comparison of sub-sampled time-series (three times a day) and real-time water discharge measurements for the 2008 year. **(b)** Distribution of the relative error for the time series presented in (a).

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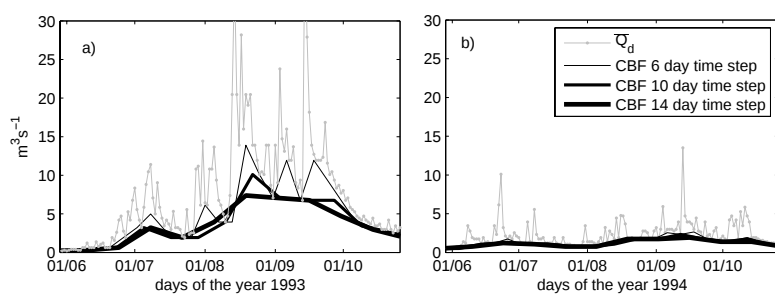


Fig. 4. Application of the hydrological separation method (Eqs. 1 and 2) for **(a)** wet year 1993 and **(b)** dry year 1994.

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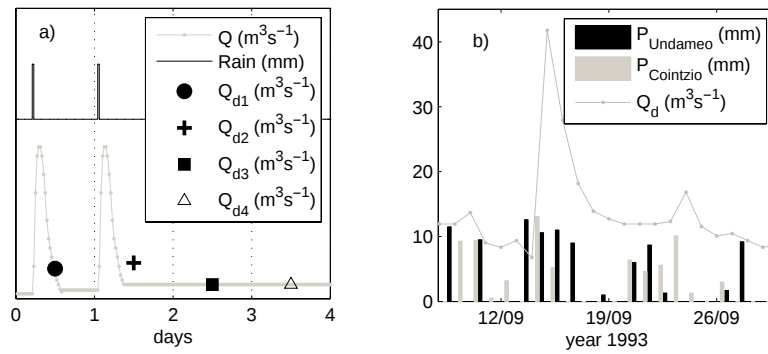


Fig. 5. Hydrograph separation when considering mean daily discharge Q_d . **(a)** Conceptual description of Eq. (3). While working with Q_d instead of Q_{inst} , a reliable estimation of baseflow (versus surface-runoff) is only possible for days without rain. In our example, Q_{d3} and Q_{d4} are the only days satisfying Eq. (3b). **(b)** Illustration through a real case. In 1993, four days of significant rains occurred in the watershed from 14–17 September (black and grey bars). It induced a strong flood event with high mean daily water discharges from 15–19 September. After this period, mean daily water discharges (grey dotted line) remained almost constant from 19–23 September, and thus equal to baseflow value.

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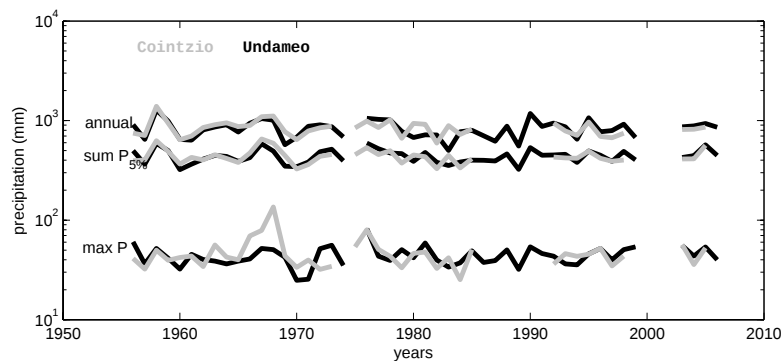


Fig. 6. Main statistical indicators of precipitation time-series at Santiago Undameo and Cointzio.

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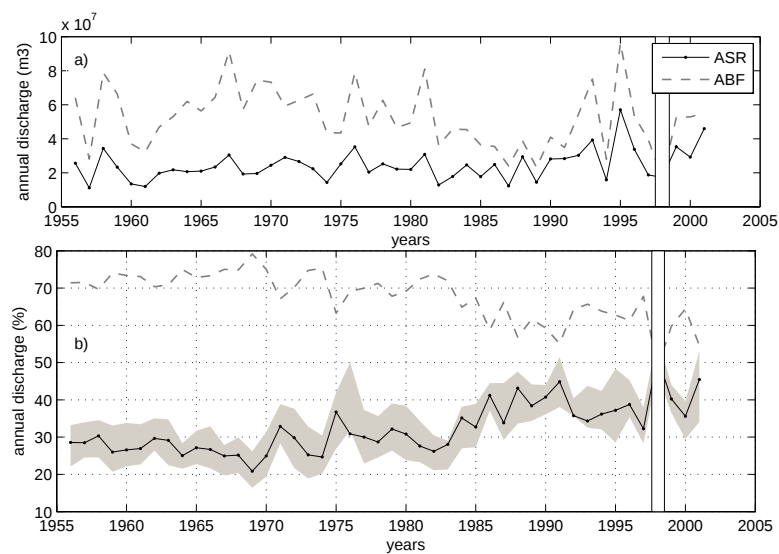


Fig. 7. Evolution of Annual Surface Runoff (ASR) and Annual Base Flow (ABF) through 1956 to 2001: **(a)** in volume, **(b)** in percentage. The grey shading in (b) shows the range of predictions given by a four and a ten days τ value.

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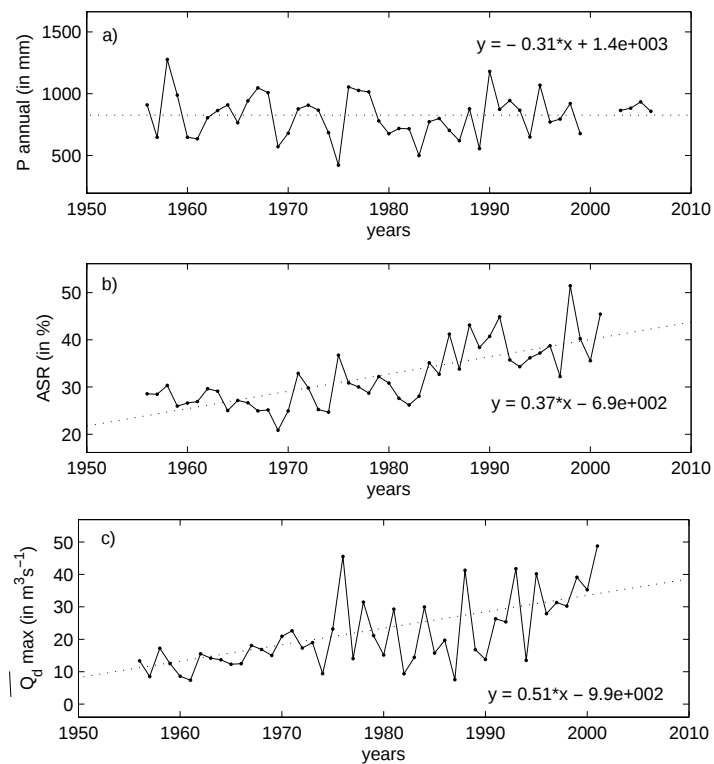


Fig. 8. Trends in precipitation, surface-runoff and extreme water discharges over the 1956–2001 period.

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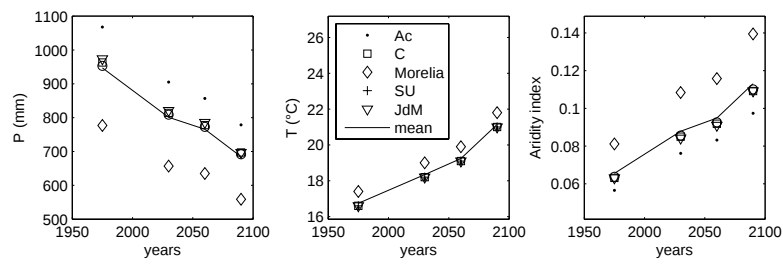


Fig. 9. (a) Estimated annual precipitation for contemporary climate (period 1961–1990, centred in 1975), and for a climate change scenario for decades centred in 2030, 2060 and 2090. **(b)** Estimated mean annual temperature for contemporary climate (period 1961–1990, centred in 1975), and for a climate change scenario for decades centred in 2030, 2060 and 2090. **(c)** Annual aridity index for contemporary climate (period 1961–1990, centred in 1975), and for a climate change scenario for decades centred in 2030, 2060 and 2090.